

Verification

&

Technical integrity

For structural design calculations

on website structolution.com

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1. Verification framework

How we ensure the accuracy of structolusion.com calculation engines.

At Structolusion, we recognize that structural safety depends on the precision of our tools. Our verification process is designed to provide engineers with the confidence that our digital outputs align perfectly with the physical requirements of the Eurocodes (EN 1990 – EN 1999).

1.1. Three tier verification process

There are three tiers of tests in the verification process. In each section the type of test will be noted.

1. **Automated audit.** We use automated suite testing or hand calculations to verify the core logic of a formula.
2. **Independent benchmark.** Results are compared against established industry benchmarks.

This includes:

- Manual hand calculations: Step-by-step verification of long-form equations.
- Peer software analysis: Comparison with known FEM solutions.
- Standardized worked examples: Verification against published books, journals and documentation.

3. **Professional review.** Final output reports are audited by a qualified Structural Engineers to ensure the logic follows standard engineering practice and provides conservative, safe results.

1.2. Precision thresholds

Every verification calculation will display the accuracy. While we strive to always stay within the tolerances, we cannot guarantee with one hundred percent certainty that precision will always be met. Structolusion.com aims for the following strict tolerances :

- **Primary values:** Variance must be $\leq 0.5\%$ for numerical calculations.
- **Empirical/iterative coefficients:** Variance of $\leq 2.0\%$ is acceptable for values involving complex iterations (e.g., lateral-torsional buckling curves or soil-structure interaction), provided the results remain conservative.

2. Analysis

2.1. Beam analysis

| | |
|--------------------|---|
| Route | https://structolution.com/calculations/beam-analysis |
| Version | 1.0.0 |
| Standard/Reference | |

Independent benchmarking

Compared with: C. Hartsuijker, H. Welleman. Mechanica: Statisch onbepaalde constructies en bezwijkanalyse. Proceedings of Boom. 2016 2nd edition. Vergeet-mij-nietjes (p405)

For the comparison the following values are used:

- $T = 7.5 \text{ kNm}$; $F = 7.5 \text{ kN}$; $q = 7.5 \text{ kN/m}$
- $EI = 210000 \text{ MPa} \times 5538400000 \text{ mm}^4$
- $l = 10000 \text{ mm}$

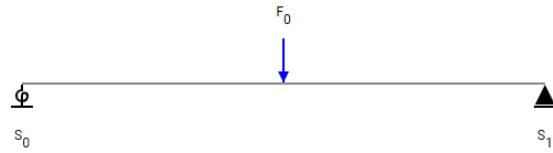
For the comparison with springs the following values are used:

- $F = 80 \text{ kN}$
- $EI = 20 \text{ MNm}^2$
- $l = 10000 \text{ mm}$
- $k = 100 \text{ kN/m}$; $k_r = 10000 \text{ kNm/rad}$

Test translational spring with Example 2.25, find vertical reaction force at S_1 with $S_{1v} = \frac{Fkl^3}{3EI+kl^3}$:



Test rotational spring, find moment M at support S_0 with $M = \frac{3Fl^2k_r}{16(3EI+k_rl)}$



| Parameter | Benchmark case | Reference value | Structolution value | Error | Status |
|--------------|------------------------------------|-----------------|---------------------|-------|--------|
| w_2 (mm) | Vergeet-mij-nietjes (1) | 18.75 | 18.76 | 0.1% | Pass |
| w_2 (mm) | Vergeet-mij-nietjes (2) | 125.0 | 125.0 | 0.0% | Pass |
| w_2 (mm) | Vergeet-mij-nietjes (3) | 468.8 | 468.8 | 0.0% | Pass |
| w_3 (mm) | Vergeet-mij-nietjes (5) | 7.813 | 7.808 | -0.1% | Pass |
| S_{1v} (N) | Example 2.25 (E=5727376MPa HEA100) | 50.00 | 50.00 | 0.0% | Pass |
| M (kNm) | Example kr (E=5727376MPa HEA100) | 93.75 | 93.75 | 0.0% | Pass |

3. Steel checks

3.1. Axial tension / compression

| | |
|--------------------|---|
| Route | https://structolution.com/calculations/steel/beam-axial |
| Version | 1.0.0 |
| Standard/Reference | EN1993-1-1:2025 |

Independent benchmark

Compared with: Silva, L.S. (2013). Eurocode 3: Design of Steel Structures. Part 1-1: General rules and rules for buildings. ECCS Eurocode Design Manuals. Example 3.3 ii (p133)

Structolution value $A_{required} = UC \times A_{applied}$,

| Parameter | Benchmark case | Reference value | Structolution value | Error | Status |
|-----------------------------------|----------------|-----------------|---------------------|-------|--------|
| $A_{required}$ (mm ²) | HEA140 | 2700 | 2700 | 0.0% | Pass |

3.2. Shear strength

| | |
|--------------------|---|
| Route | https://structolution.com/calculations/steel/beam-shear |
| Version | 1.0.0 |
| Standard/Reference | EN1993-1-1:2025 |

Independent benchmark

Compared with: Silva, L.S. (2013). Eurocode 3: Design of Steel Structures. Part 1-1: General rules and rules for buildings. ECCS Eurocode Design Manuals. Example 3.4 iv (p144); Example 3.5 iv (p149)

| Parameter | Benchmark case | Reference value | Structolution value | Error | Status |
|------------------|-------------------------|-----------------|---------------------|-------|--------|
| $V_{pl,Rd}$ (kN) | Example 3.4 HEA220 S235 | 280.4 | 280.4 | 0.0% | Pass |
| $V_{pl,Rd}$ (kN) | Example 3.5 HEA360 S275 | 777.3 | 777.2 | 0.0% | Pass |

3.3. Bending moment strength

| | |
|--------------------|---|
| Route | https://structolution.com/calculations/steel/beam-bending |
| Version | 1.0.0 |
| Standard/Reference | EN1993-1-1:2025 |

Independent benchmark

Compared with: Silva, L.S. (2013). Eurocode 3: Design of Steel Structures. Part 1-1: General rules and rules for buildings. ECCS Eurocode Design Manuals. Example 3.4 ii (p143); Example 3.5 ii (p148)

Structololution value $W_{y,required} = UC \times W_{y,applied}$, hence the +/- 0.5 percent error. The section modulus at Structololution for RHS profiles are a bit smaller than the reference material. Nevertheless it is shown that elastic and plastic bi-axial bending corresponds well with the reference material.

| Parameter | Benchmark case | Reference value | Structololution value | Error | Status |
|-------------------------------------|--|-----------------|-----------------------|-------|--------|
| $W_{y,applied}$ (cm ³) | Example 3.4 HEA220 S23 | 568.5 | 568.4 | 0.0% | Pass |
| $W_{y,required}$ (cm ³) | Example 3.4 HEA220 S235 | 446.8 | 449.036 | 0.5% | Pass |
| $W_{y,applied}$ (cm ³) | Example 3.4 IPE270 S235 | 484 | 483.9 | 0.0% | Pass |
| $W_{y,required}$ (cm ³) | Example 3.4 IPE270 S235 | 446.8 | 445.2 | -0.4% | Pass |
| $W_{y,applied}$ (cm ³) | Example 3.5 HEA360 S275 | 2088 | 2088 | 0.0% | Pass |
| $W_{y,required}$ (cm ³) | Example 3.5 HEA360 S275 | 2036.4 | 2046.24 | 0.5% | Pass |
| $W_{Pl,y}$ (cm ³) | Example 3.6 RHS200x100x8 (hot finished) S275 | 286 | 282 | -1.4% | Pass |
| $W_{Pl,z}$ (cm ³) | Example 3.6 RHS200x100x8 (hot finished) S275 | 174 | 171.8 | -1.3% | Pass |
| $M_{Pl,Rd,y}$ (kNm) | Example 3.6 RHS200x100x8 (hot finished) S275 | 78.7 | 77.55 | -1.5% | Pass |
| $M_{Pl,Rd,z}$ (kNm) | Example 3.6 RHS200x100x8 (hot finished) S275 | 47.9 | 47.25 | -1.4% | Pass |
| UC double bending | Example 3.6 RHS200x100x8 (hot finished) S275 | 0.83 | 0.85 | 2.4% | Pass |
| $W_{El,y}$ (cm ³) | Example 3.6 RHS250x150x6.3 (hot finished) S275 CC3 | 334 | 331.4 | -0.8% | Pass |
| $W_{El,z}$ (cm ³) | Example 3.6 RHS250x150x6.3 (hot finished) S275 CC3 | 252 | 249.9 | -0.8% | Pass |
| UC double bending | Example 3.6 RHS250x150x6.3 (hot finished) S275 CC3 | 0.91 | 0.92 | 0.8% | Pass |

3.4. Column buckling

| | |
|--------------------|---|
| Route | https://structololution.com/calculations/steel/beam-flexural-buckling |
| Version | 1.0.0 |
| Standard/Reference | EN1993-1-1:2025 |

Independent benchmark

Compared with: Silva, L.S. (2013). Eurocode 3: Design of Steel Structures. Part 1-1: General rules and rules for buildings. ECCS Eurocode Design Manuals. Example 3.9 (p188); Example 3.10 (p191)

| Parameter | Benchmark case | Reference value | Structololution value | Error | Status |
|-------------|--|-----------------|-----------------------|-------|--------|
| N_b,Rd (kN) | Example 3.9 HEB240 S355 | 1618 | 1624 | 0.4% | Pass |
| N_b,Rd (kN) | Example 3.10 SHS120x8 (hot finished) S275 | 835.7 | 826.6 | -1.1% | Pass |
| N_b,Rd (kN) | Example 3.10 SHS80/6.3 (hot finished) S275 | 398.2 | 392 | -1.6% | Pass |
| N_b,Rd (kN) | Example 3.10 HEA180 S275 | 851.2 | 852.3 | 0.1% | Pass |

3.5. Lateral torsional buckling

| | |
|--------------------|---|
| Route | https://structolition.com/calculations/steel/beam-lateral-torsional-buckling |
| Version | 1.0.0 |
| Standard/Reference | EN1993-1-1:2025 |

Independent benchmark

Compared with: Silva, L.S. (2013). Eurocode 3: Design of Steel Structures. Part 1-1: General rules and rules for buildings. ECCS Eurocode Design Manuals. Example 3.11 (p214);

Structolition value for Example 3.11 received with custom LTB values for the unrestrained loading:

- Free standing load with $C_2=0.42$
- Custom C_1 factor $C_1=1.04$

The critical bending moment M_{cr} matches. The used reference uses different α_{LT} values based on the Generation 1 Eurocode, explaining the difference in the moment LTB resistance $M_{b,Rd}$.

| Parameter | Benchmark case | Reference value | Structolition value | Error | Status |
|------------------|--|-----------------|---------------------|-------|--------|
| $M_{b,Rd}$ (kNm) | Example 3.11 HEA240 S235 unrestrained | 131.2 | 129.2 | -1.5% | Pass |
| M_{cr} (kNm) | Example 3.11 HEA240 S235 unrestrained | 231.5 | 232.5 | 0.4% | Pass |
| $M_{b,Rd}$ (kNm) | Example 3.11 HEA220 S235 unrestrained using less conservative method | 101.5 | 94.97 | -6.4% | Pass |
| M_{cr} (kNm) | Example 3.11 HEA220 S235 unrestrained using less conservative method | 158.8 | 158.9 | 0.1% | Pass |
| $M_{b,Rd}$ (kNm) | Example 3.11 HEA220 S235 restrained | 124.2 | 116.3 | -6.4% | Pass |
| M_{cr} (kNm) | Example 3.11 HEA220 S235 restrained | 551.3 | 551.5 | 0.0% | Pass |

3.6. Fillet weld

| | |
|--------------------|---|
| Route | https://structolition.com/calculations/fillet-weld-check |
| Version | 1.0.0 |
| Standard/Reference | EN1993-1-8:2008 (material following EN:2025) |

Independent benchmark

Comparison with: Gresnigt, A.M. (2014). Design Rules for Fillet Welds in Eurocode 3 and AISC. Proceedings of EUROSTEEL 2014, Naples, Italy.

Each case is checked for $\sigma_x = f_y$ from Table 1 of the compared material. The other selected parameters in the calculations are 'weld length reduction' off, β_{Lw1} and a 'custom material' with $\gamma_{M2} = 1.25$.

All calculations pass except for S420, which has less capacity at Structolotion. Table 1 from the reference suggests that $a=0.75t$. Formula (5) from the reference gives $a=0.714t$, which the Structolotion calculation perfectly matches.

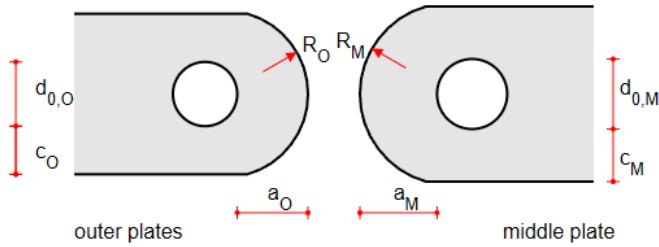
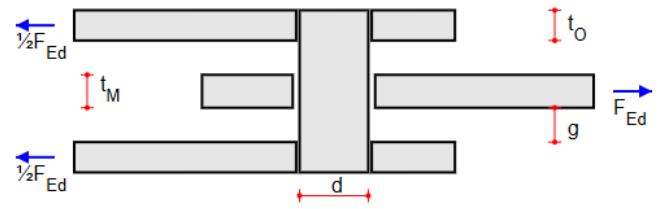
| Parameter | Benchmark case | β_w (-) | f_u (MPa) | Reference value | Structolotion value | Error | Status |
|------------------------|---|------------------|-------------|--------------------|------------------------|-------|--------|
| UC_{VM} | S235/S235W, $t \leq 40\text{mm}$, $a=0.461t$ | 0.8 | 360 | 1.00 | 1.00 | 0% | Pass |
| UC_{VM} | S355/S355W, $t \leq 40\text{mm}$, $a=0.553t$ | 0.9 | 510 | 1.00 | 1.00 | 0% | Pass |
| UC_{VM} | S355 N/NL, $t \leq 40\text{mm}$, $a=0.576t$ | 0.9 | 490 | 1.00 | 1.00 | 0% | Pass |
| UC_{VM} | S355 M/ML, $t \leq 40\text{mm}$, $a=0.602t$ | 0.9 | 470 | 1.00 | 1.00 | 0% | Pass |
| UC_{VM} | S420 N/NL/M/ML, $t \leq 40\text{mm}$, $a=0.714t$ | 1.0 | 520 | 1.05 | 1.00 | -5% | Pass |
| UC_{VM} | S460 N/NL/M/ML, $t \leq 40\text{mm}$, $a=0.754t$, | 1.0 | 540 | 1.00 | 1.00 | 0% | Pass |

3.7. Clevis joint

| | |
|--------------------|---|
| Route | https://structolotion.com/calculations/clevis-joint-check |
| Version | 1.0.0 |
| Standard/Reference | EN1993-1-8:2025 |

Independent benchmark

Comparison with manual hand calculation. The engine results were compared against a step-by-step hand calculation following the Eurocode equations directly.



| Pin | Middle Plate | Outer Plate |
|----------------------------|----------------------------|----------------------------|
| $d = 10 \text{ mm}$ | $t_M = 4.8 \text{ mm}$ | $t_0 = 4.41 \text{ mm}$ |
| $g = 5 \text{ mm}$ | $d_{0,M} = 12 \text{ mm}$ | $d_{0,O} = 11 \text{ mm}$ |
| $f_y = 355 \text{ N/mm}^2$ | $c_M = 9 \text{ mm}$ | $c_0 = 8.25 \text{ mm}$ |
| $f_u = 460 \text{ N/mm}^2$ | $a_M = 13.2 \text{ mm}$ | $a_0 = 12.11 \text{ mm}$ |
| | $R_M = 15.6 \text{ mm}$ | $R_O = 14.3 \text{ mm}$ |
| | $f_y = 275 \text{ N/mm}^2$ | $f_y = 235 \text{ N/mm}^2$ |

$$F_{Ed} = 10 \text{ kN}$$

$$F_{Ed,ser} = 5 \text{ kN}$$

$$\gamma_{M0} = 0.9$$

$$\gamma_{M2} = 1.25$$

$$\gamma_{M6,ser} = 1.11$$

Using custom γ_M values. The geometry of the eyes in the middle and outer plate are checked following Table 5.2 Type A. In case of setting 'geometry by the hole diameter' then the distance a and distance c are the smallest of Type A and Type B.

| Parameter | Reference value | Structoluation value | Error | Status |
|------------------------------|-----------------|----------------------|-------|--------|
| UC pin shear | 0.29 | 0.29 | 0% | Pass |
| UC pin bending | 0.72 | 0.72 | 0% | Pass |
| UC pin shear + bending | 0.61 | 0.61 | 0% | Pass |
| UC middle plate bearing | 0.45 | 0.45 | 0% | Pass |
| UC outer plate bearing | 0.29 | 0.29 | 0% | Pass |
| UC geometry middle plate a | 0.86 | 0.86 | 0% | Pass |
| UC geometry middle plate c | 0.82 | 0.82 | 0% | Pass |
| UC geometry outer plate a | 0.78 | 0.79 | 0% | Pass |
| UC geometry outer plate c | 0.71 | 0.71 | 0% | Pass |
| UC pin bending SLS | 0.84 | 0.84 | 0% | Pass |
| UC middle plate bearing SLS | 0.70 | 0.70 | 0% | Pass |
| UC outer plate bearing SLS | 0.45 | 0.45 | 0% | Pass |
| UC middle plate hertz stress | 2.00 | 2.00 | 0% | Pass |
| UC outer plate hertz stress | 1.22 | 1.22 | 0% | Pass |